

SEAMOCS Workshop

Implication of climate change for marine and coastal safety



Waves, wave climate, extreme waves – knowledge from direct observations, space-born retrievals, and modelling

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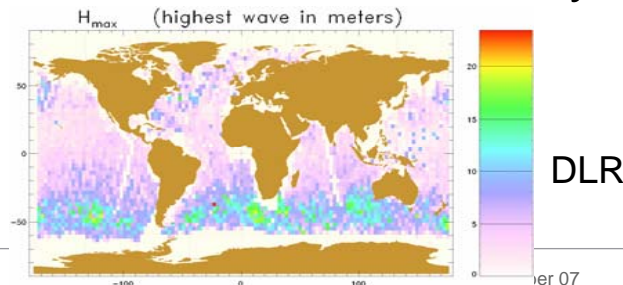
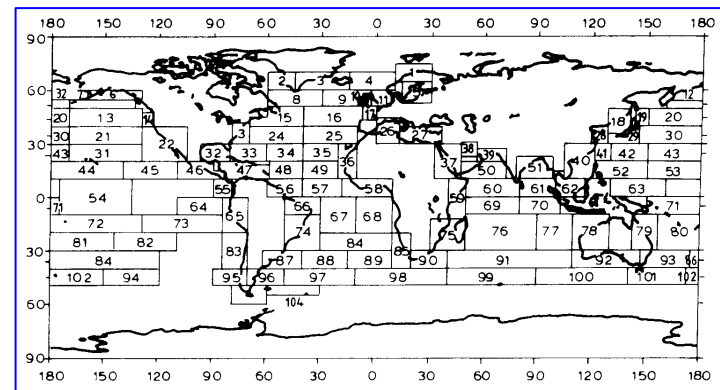


Existing Wave Data

- Need for **improving the availability, quality and reliability** of **environmental databases** for marine structures' design & operational criteria - **international professional organisations, Classification Societies** and **offshore companies**.
- **High uncertainty** - **over-design** or **under-design** of marine structures' → significant **economic/risk impact**, e.g. Guedes Soares and Trovão (1991), and Bitner-Gregersen et al. (1995).

- **Wave data**

- instrumental; location specific
- *Global Wave Statistics (GWS)*
- numerical generated data calibrated by in-situ & satellite data
- satellite data



Implication of climate change for marine and coastal safety

Questions:

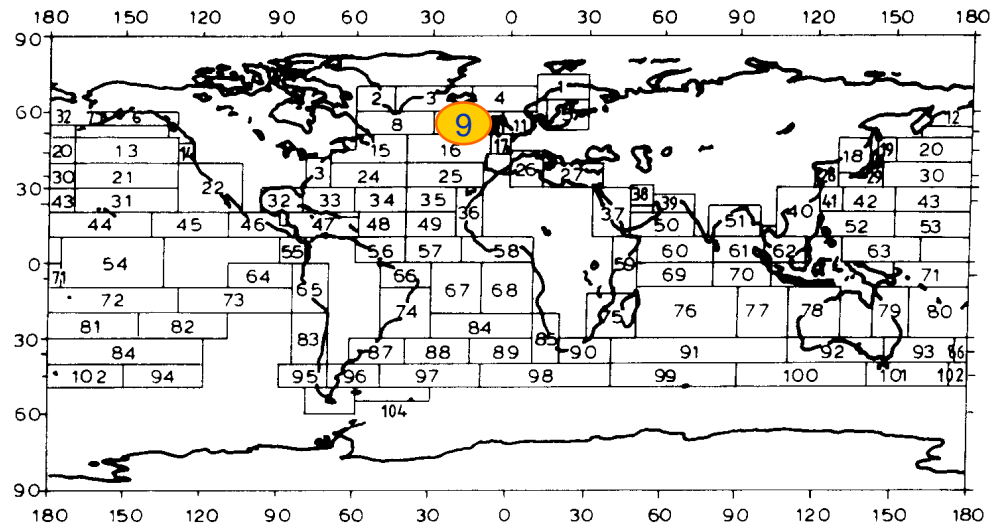
- Is observed climate change a real trend or is it a result of climatic variations in the nature?
- How reliable are wave observations collected in the last 1-2 decades?
- What is the accuracy of numerical wave climate models?

- How consistence are predictions of the existing global wave databases ?
- Can the existing global wave databases be utilized fully by the industry?

The results presented were partly developed in collaboration with Prof. Carlos Guedes Soares.

Global Wave Databases

- **HIPOCAS** (Guedes Soares et al. 2002) - period **(1958 – 2002)**
- **Fugro OCEANOR** (Barstow et al. 2003) - period **(1984-2003)**
- **ARGOSS** (Valk et al. 2004) - period **(1990-2002)**
- **Global Wave Statistics (GWS)**, Hogben et al. (1986) - period **(1949-1986)**
- **SBWR data** (Draper and Whitaker 1965) – period **(1952-1964)**
- **Altimeter data** - period **(1990-2002)**



Average Wave (Sea State) Steepness

- Important characteristic of a sea state
- Critical parameter for the design of marine structures

Guedes Soares, C.; Bitner-Gregersen, E., and Antão, P. (2001), “Analysis of the **Frequency of Ship Accidents** under Severe **North Atlantic Weather Conditions**” ; U.Leuven (2003)

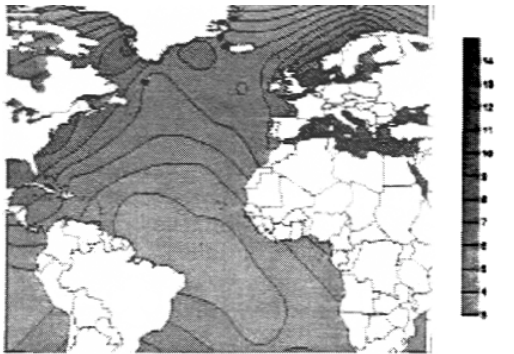


Figure 3 - Distribution of 20-years extreme wave steepness (data from Bitner Gregersen et al, 1995)

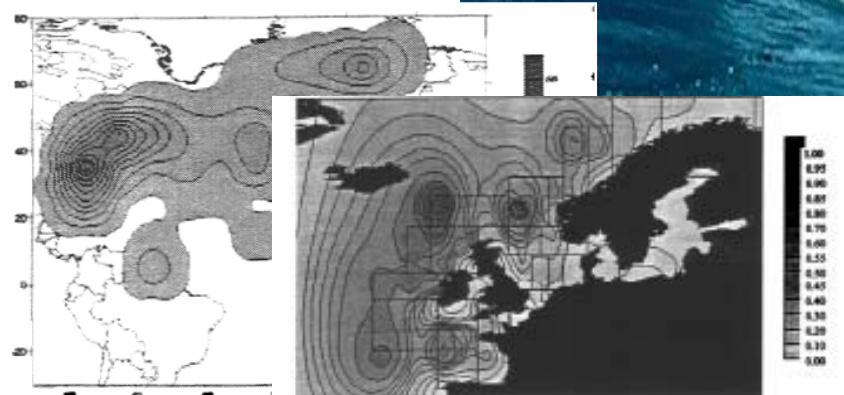


Figure 16 – Geographical distribution of the frequency of weather related accidents (n=355)

Average Wave Steepness

Average wave steepness is usually defined as

$$S_t = 2\pi H_s / gT_z^2$$

Corresponding the **N-year steepness** is

$$S_t = 2\pi H_{s_N} / gT_{z_N}^2$$

For **design purposes** **extreme values** of the **average wave steepness** are of particular interest. May be determined:

- Existing standards
- Empirical expressions
- Theoretical formulas
- Directly from wave data.



Average Wave Steepness

An **upper limit of the average wave steepness** based on **field data** and **theoretical models**:

- **Carter (1982)** for a **Pierson-Moskowitz spectrum** the steepness was **1/19.7**.
- **Carter (1982)** for **growing seas** in open waters for which the steepness was generally **higher** than about **1/16**. **For limited fetches** the waves were considerably steeper, in **the range 1/11.1 – 1/14.8**. Confirmed by Battjes (1972) and Draper (1976).
- **Bjerke et al. (1990), Mathiesen and Torsethaugen (1992), and Torsethaugen (1993), Bitner-Gregersen et al. (1998)** for the Norwegian Continental Shelf in **the range 1/15 – 1/14.3**.
- **DNV Rules (DNV 2000)** allow **$S_{Tz} = 1/15$** for high sea states, for lower **$S_{Tz} = 1/10$** .
- **UK Department of Energy (1990)** **$S_{Tz} = 1/16 - 1/20$** .

Average Wave Steepness

- **Extreme wave steepness** is calculated by **fitting the data** by a **joint environmental model** due Bitner-Gregersen the 3-parameter Weibull distribution for **significant wave height**

$$f_{H_{m0}|\varpi}(h | \theta_i) = \frac{\beta}{\alpha} \left(\frac{h - \gamma}{\alpha} \right)^{\beta-1} \exp \left(- \left(\frac{h - \gamma}{\alpha} \right)^\beta \right)$$

where α = scale parameter, β = slope parameter, γ = location parameter, and ϖ is the main wave direction, and the conditional (on H_{m0}) lognormal distribution for **zero-crossing wave period**

$$f_{T_z|H_{m0},\Theta}(t | h, \theta_i) = \frac{1}{\sqrt{2\pi\sigma} \cdot t} \exp \left[- \frac{(\ln t - \mu)^2}{2\sigma^2} \right]$$

$$\mu = E(\ln T_z) = a_1 + a_2 h^{a_3}$$

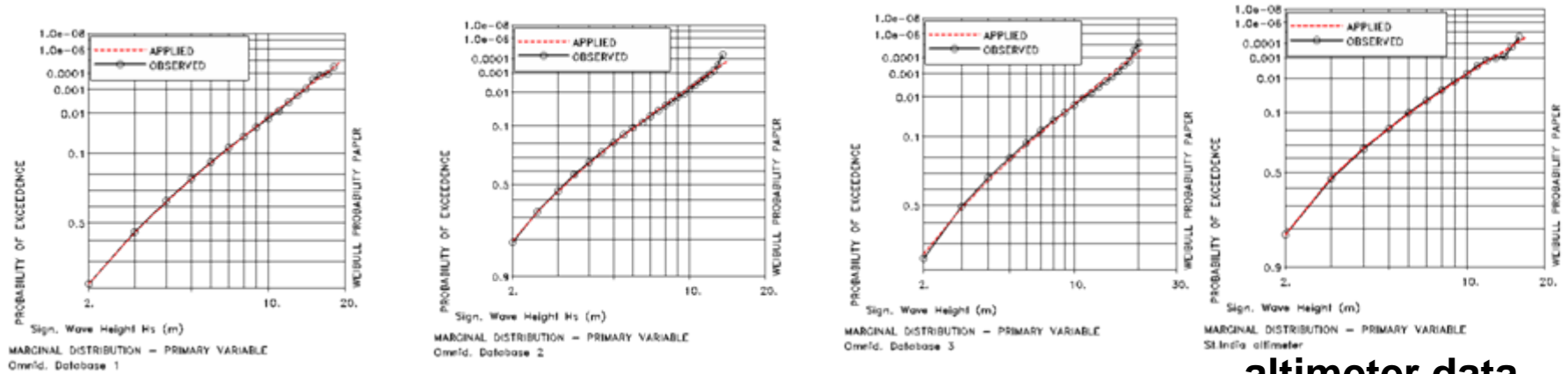
$$\sigma = \text{Std}(\ln T_z) = b_1 + b_2 h^{b_3}$$

N-year characteristic largest value of H_{m0}

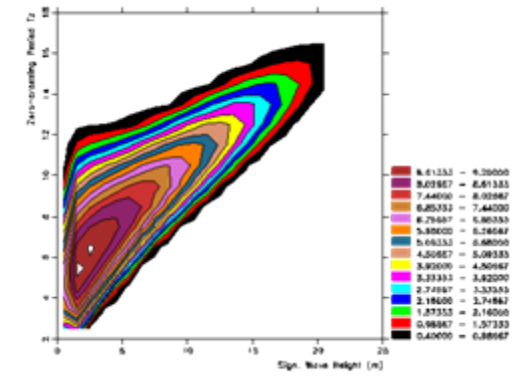
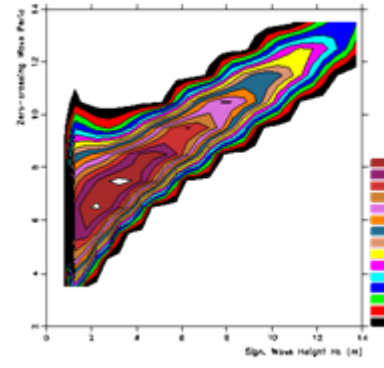
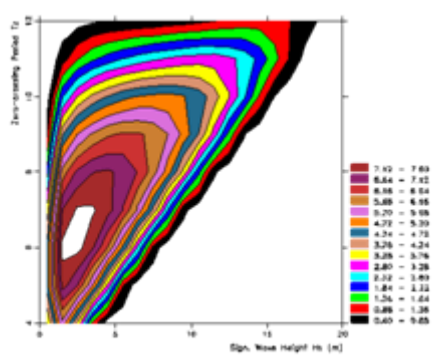
$$h_{m0N} = \gamma + \alpha (\ln N)^{1/\beta}$$

Extreme Average Wave Steepness

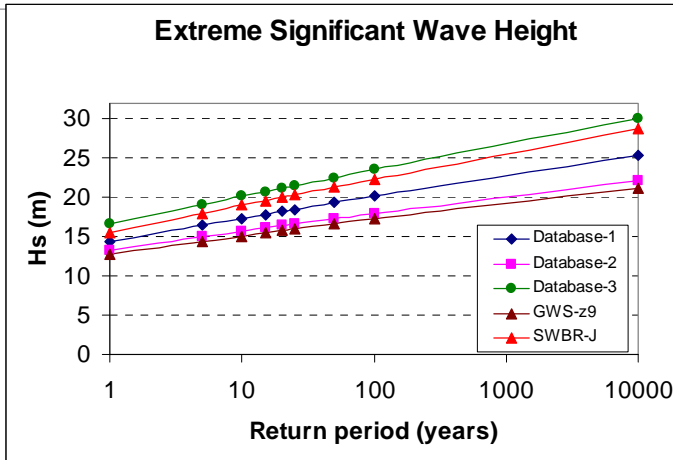
● Fit of the joint model



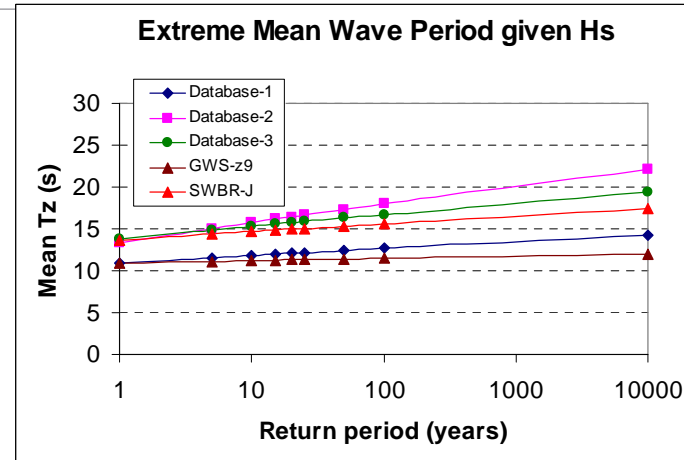
altimeter data



Extreme Average Wave Steepness



Extreme N-year H_{m0}



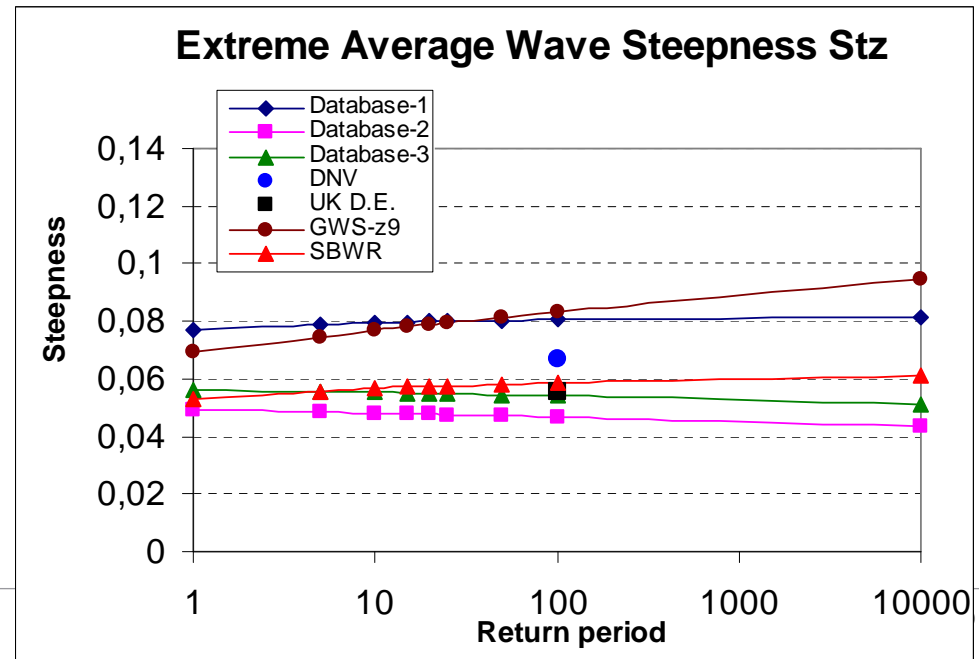
Extreme T_{m02} given N-year H_{m0}

13-year observations

Date	Altimeter	H_s (m)
19930120	224900	14,8
19930120	224900	15,8
19930120	224900	16,2
19930120	224900	14,1

$H_{s100} = 19.6\text{m}$

N-year extreme wave steepness



Environmental contours, Winterstein et al. (1993) MANAGING RISK

- The contours are found from the (H_s, T_z) dist. by relating H_s and T_z (or T_p) to **standard normal variables** U₁ and U₂

$$h_s = F_{H_s}^{-1}(\Phi(u_1)) \quad t_z = F_{T_z|H_s}^{-1}(\Phi(u_2))$$

where $\Phi(\cdot)$ is the standard normal distribution function.

- These equations give a **contour with return period 100 years** by varying U₁ and U₂ along the circle

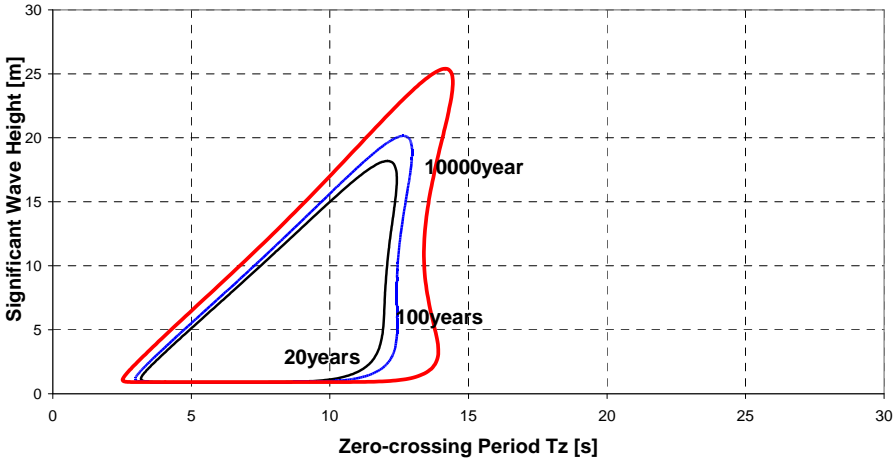
$$\sqrt{u_1^2 + u_2^2} = \beta$$

- where $\beta = \Phi^{-1}(1 - p_f) = \Phi^{-1}\left(1 - \frac{1}{100 \cdot 365 \cdot 8}\right) = 4.5$

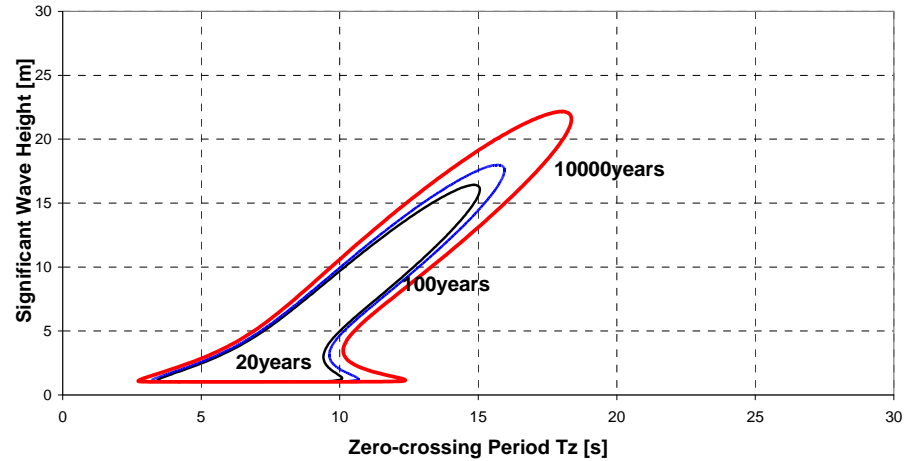
See DNV Offshore standards (2001).

Environmental Contours

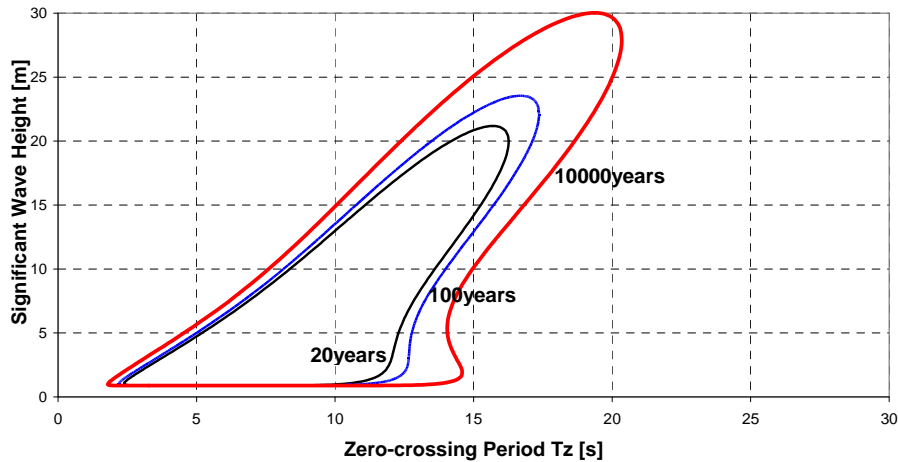
Environmental contours, Data base 1



Environmental contours, Data base 2



Environmental contours, Data base 3



Extreme Average Wave Steepness

Uncertainties as suggested by Bitner-Gregersen and Hagen:

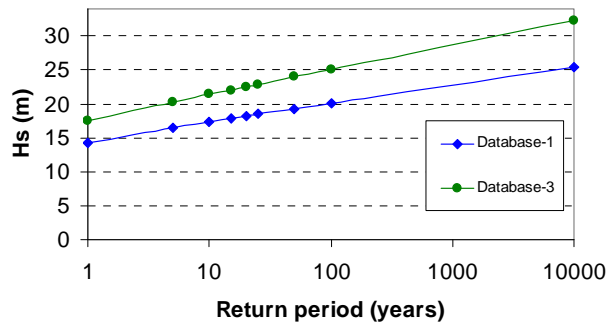
- Data uncertainty
- Statistical uncertainty (sampling variability, fitting procedure)
- Model uncertainty (adopted model to fit the data)
- Climatic variations (different time periods which the data sources cover as well as different locations they represent).

As the **same joint model** as well as the **fitting procedure** has been used to fit the data the **discrepancies** of the **results** can **primarily be explained** by the **data** and **climatic uncertainty**.

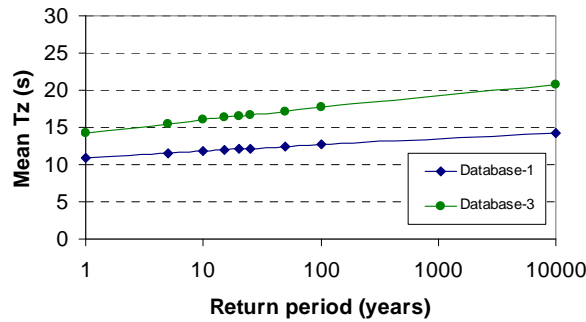
It was expected to see some **convergence of the extreme wave steepness predictions.**

Extreme Average Wave Steepness

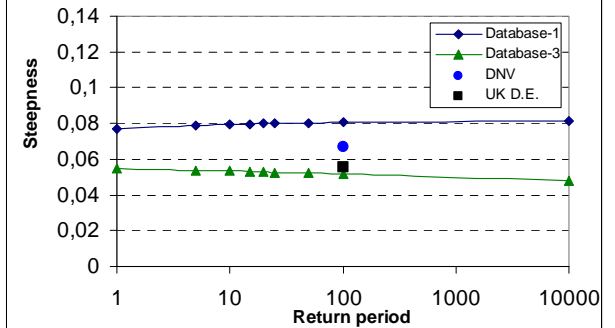
Extreme Significant Wave Height



Extreme Mean Wave Period given H_s



Extreme Average Wave Steepness S_{Tz}

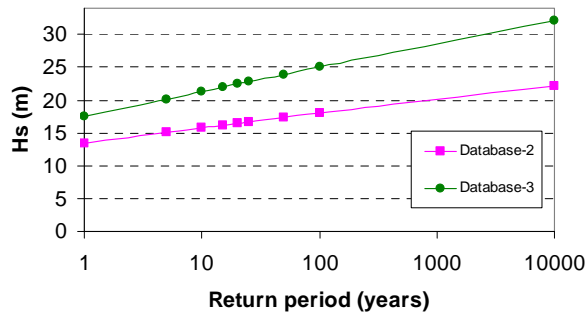


Extreme N-year H_{m0}

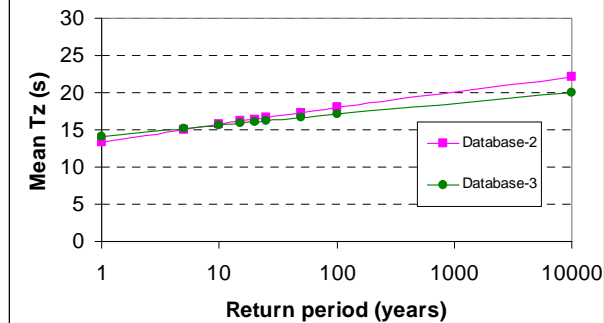
Extreme T_{m02} given N-year H_{m0}

Extreme steepness

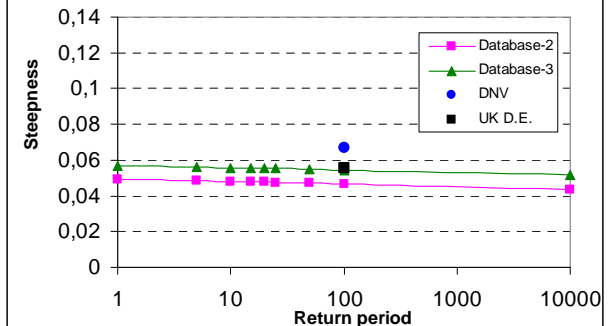
Extreme Significant Wave Height



Extreme Mean Wave Period given H_s



Extreme Average Wave Steepness S_{Tz}



Extreme N-year H_{m0}

Extreme T_{m02} given N-year H_{m0}

Extreme steepness

Discrepancies - $H_{m0} : \cong 5.5m$, $T_{m0} : \cong 4s$, $S_{Tz} : \cong 33\%$

Conclusions

- There is still **need** for **further discussion** of **accuracy** of the recently developed **databases** and **uncertainties related** to them before they can be **fully utilized in engineering applications**.
- The **extremes predicted** by **three** considered herein **databases differ significantly**.
- Although given in the literature comparison of time series predicted by the databases and measured data show good agreement, **uncertainties** involved seem to **accumulate** resulting in **large discrepancies in extreme statistics**.





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